

Client: [REDACTED] Report No. 200001-CR-000
 Title: [REDACTED] Indoor Sports Hall Roof

Project: [REDACTED] Sports Hall Roof

Method: See Within

Acceptance Criteria: See Within

Remarks:

Verification Method Design Review Method Alternate Calculation Qualification Test
 Other No Verification Necessary

Results: See body of calculation

Computer Programs Used	Program Name	Version/Revision	License
		CEDRUS-8	1.00

REVISIONS

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REPORT
COVER
SHEET



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1. GENERAL PRINCIPLE OF CALCULATIONS

1.1 Slab Design

Indoor Sports Hall Roof Slab extends 18 m in Y direction and 32 m in X direction. The slab has to support loads within a limited thickness of 60 cm.

The slab is reinforced with post-tensioned bonded tendons in y direction and rebars in both orthogonal directions. In the scope of this work slab model developed considering only the region designed with prestressed concrete. The other interior slabs and supporting columns are not included in the scope.

1.2 Main Data

1.2.1 Codes

The calculations are made according to the [REDACTED]

Other references and literature on post-tensioned floors currently used are:

- [REDACTED] SIA (Technical rules for the design and the execution of prestressed concrete at ultimate limit state)
- American code ACI 318 (Building code requirements for reinforced concrete)
- "Design of prestressed concrete structures", by Prof. T. Y. LI, (Wiley ed.)
- Several publications from the PTI (Post-Tensioning Institute)

1.2.2 Materials

Concrete: $f_{c28} = 30 \text{ N/mm}^2$ at 28 days (characteristic strength at 28 days on cylinder)

Tendons: T 15 Z, grade 1860, section 150 mm^2
Initial jacking force: 209 kN (0.75fpu)

Reinforcement Steel: $f_y = 420 \text{ N/mm}^2$

1.2.3 Loading

Own weight calculated with specific weight of concrete 25 kN/m^3

Superimposed dead loads: 1.00 kN/m^2

Live loads: 5.0 kN/m^2

1.2.4 Post-tensioning

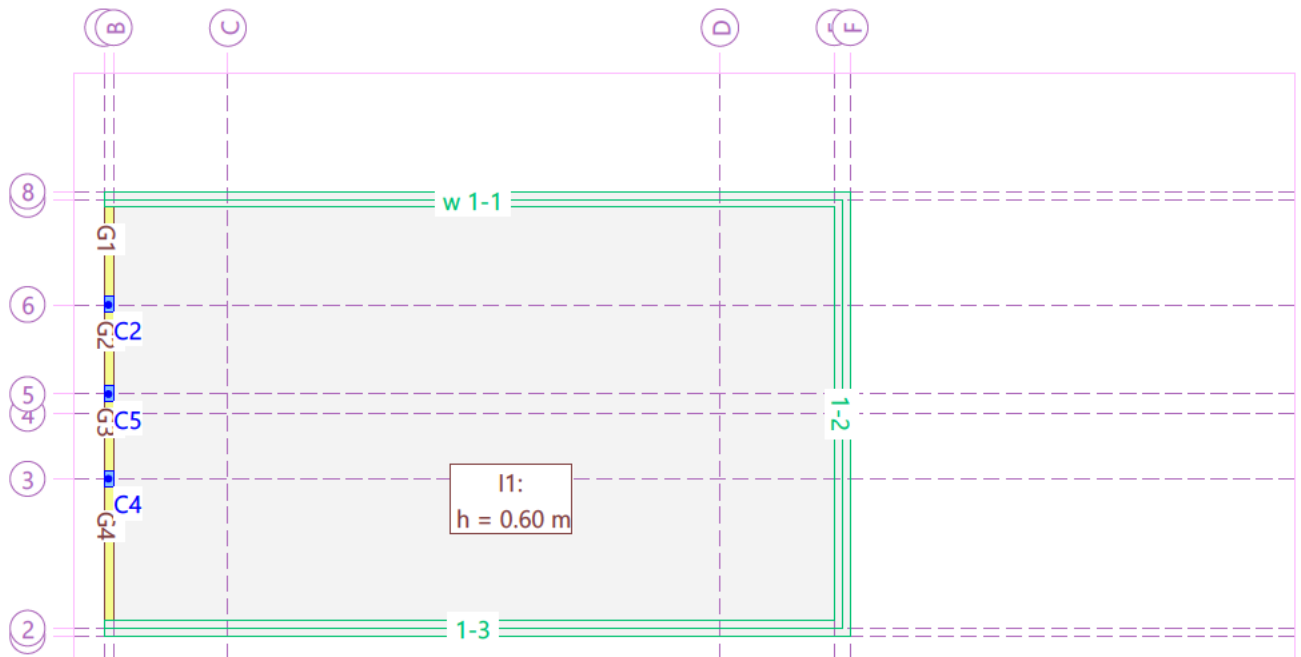
Post-tensioning is introduced as an external force, according to the load-balancing method described by T.Y.LIN.

Profiles of the tendons are parabolic and linear.

Losses due to elastic shortening, shrinkage and creep of concrete, friction of cables, steel relaxation and anchorage sets are taken into account. The calculations of the losses are made according to the Swiss code SIA.

STRUCTURE

Scale 1:330.7



STRUCTURE DATA

MATERIALS Code: SIA

ID	Type	Member	E [kN/mm ²]	v	ρ [t/m ³]	α [‰]	Class
CS	Concrete	Slab	36.40	0.17	2.50	0.010	C40/50
R	Reinforcement ste	(general)	205.00	0.30	8.00	0.012	B500B
P	PT Steel	(general)	195.00	0.30	8.00	0.012	Y1860

MATERIAL Concrete

ID	Class	$-\eta f_c f_{ck}$ [N/mm ²]	E_{cm} [kN/mm ²]	f_{ctm} [N/mm ²]	τ_{cd} [N/mm ²]	$f_{ck, cube}$ [N/mm ²]
CS	C40/50	-36.30	36.40	3.50	1.25	50.00

MATERIAL Reinforcement steel

ID	Class	$-f_{sk}$ [N/mm ²]	E_s [kN/mm ²]	f_{sk} [N/mm ²]	ϵ_{ud} [‰]	$k_s f_{sk}$ [N/mm ²]	Ductility [m]
R	B500B	-500.00	205.00	500.00	45.0	540.00	high ductility

MATERIAL PT Steel

ID	Class	$-f_{p0.1k}$ [N/mm ²]	E_s [kN/mm ²]	$f_{p0.1k}$ [N/mm ²]	ϵ_{ud} [‰]	f_{pk} [N/mm ²]
P	Y1860	-1600.00	195.00	1600.00	20.0	1860.00

MATERIAL BOXES: Isotropic

Id	Geometry		f_E	Materials	
	Slab thickness [m]	Top surface level [m]		Body	Reinforcement
I1	0.60	0	1.000	CS	R

MATERIAL BOXES: Distance to edge and basic reinforcement

Id	Reinforcement cover				Basic reinforcement			
	u_{XT} [cm]	u_{YT} [cm]	u_{XB} [cm]	u_{YB} [cm]	a_{sXT} [cm ² /m]	a_{sYT} [cm ² /m]	a_{sXB} [cm ² /m]	a_{sYB} [cm ² /m]
I1	3.0	3.0	3.0	3.0	-	-	-	-